



## The Stability Analysis in Geotechnical Engineering by Rigid Plastic Finite Element Method

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### ● Research Outline

#### Bearing capacity Analysis

My research is applying the Rigid Plastic Finite Element Method (RPFEM) to two type of stability problems in geotechnical engineering. RPFEM is based on the upper boundary method in the limit theorem. Those stability problems which we take as an object of study are the bearing capacity problem and slope stability problem in soils.

First, we calculate the ultimate bearing capacity of the soft clay ground in which the rigid continuous footing foundation is installed with penetration by RPFEM (Fig. 1).

The calculation results are shown in Fig. 2. Under this calculation, if embedded depth  $D_f$  is same although footing foundation width  $B$  is different, bearing capacity  $q_f$  is almost same value.

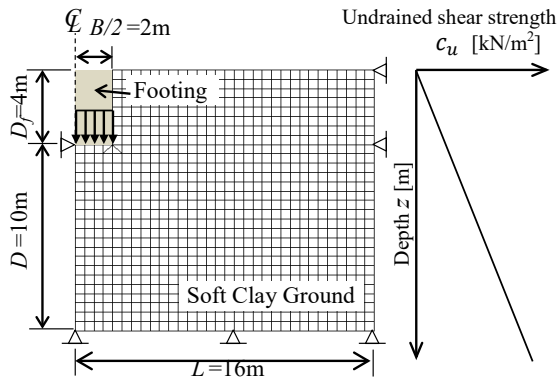


Fig. 1. Finite element mesh and boundary conditions

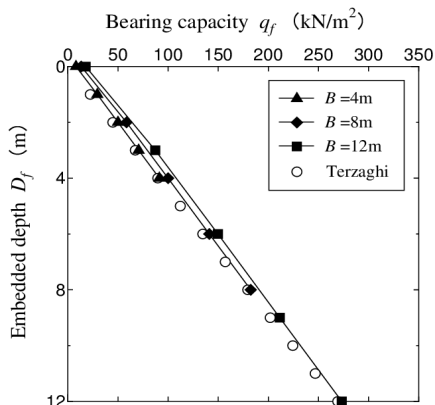


Fig. 2. Relationship between  $q_f$  and  $D_f$

#### Slope stability Analysis

We try to calculate to slope stability problem by RPFEM. The purpose of this analysis is to know the influence of material constants ( $\gamma_t$  and  $c_u$ ) and shape of embankment (height of embankment  $H$  and inclination of slope  $\theta$  in Fig. 3.) to the safety factor  $F_s$ .

Fig. 4 shows relationship between  $\gamma' H/c_u$  and safety factor  $F_s$ . It found that both of relationship express only one curve like a hyperbola every each inclination of slope  $\theta$ . Thus, the findings of this calculation is we can get safety factor  $F_s$  easily once inclination of slope  $\theta$  and height  $H$  of embankment is decided after we experience the test for soil wet density and shear test under undrained condition in laboratory. Moreover, we shall pay attention to the line which safety factor  $F_s = 1.0$ . Once we get the value of  $\gamma_t$ ,  $c_u$ , and  $\theta$ , we can estimate critical height of embankment  $H_c$ .

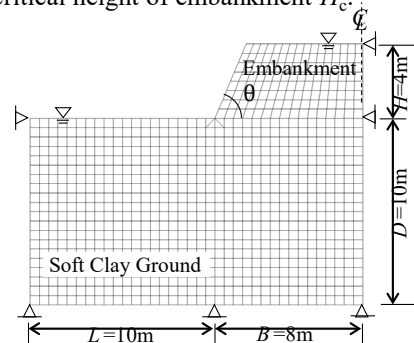


Fig. 3. Finite element mesh and boundary conditions

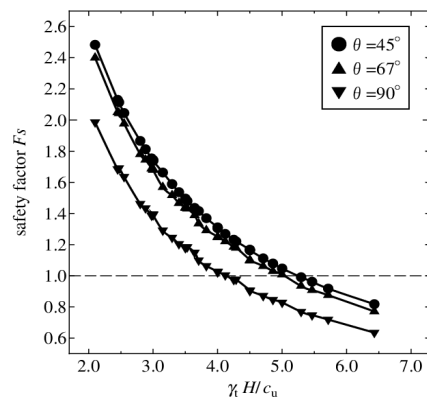


Fig. 4. Influence of material constant and shape of embankment